Vertical Evacuation Structures ASCE 7-22 Tsunami Loads and Effects

Ian N. Robertson, S.E., ianrob@hawaii.edu Arthur N.L. Chiu Distinguished Professor of Structural Engineering, UH Manoa Tsunami Loads and Effects Subcommittee member Sept 14, 2016 – ITIC Seminar - Honolulu

Tohoku Tsunami photograph at Minami Soma by Sadatsugu Tomizawa

Outline

- Need for Vertical Evacuation Refuges
 - Performance of Vertical Evacuation Refuges during Tohoku Tsunami
- FEMA P-646 design guidelines
 - **ASCE-7 Tsunami Loads and Effects chapter**
- Vertical Evacuation Refuge designs in US
- Conclusions

Outline

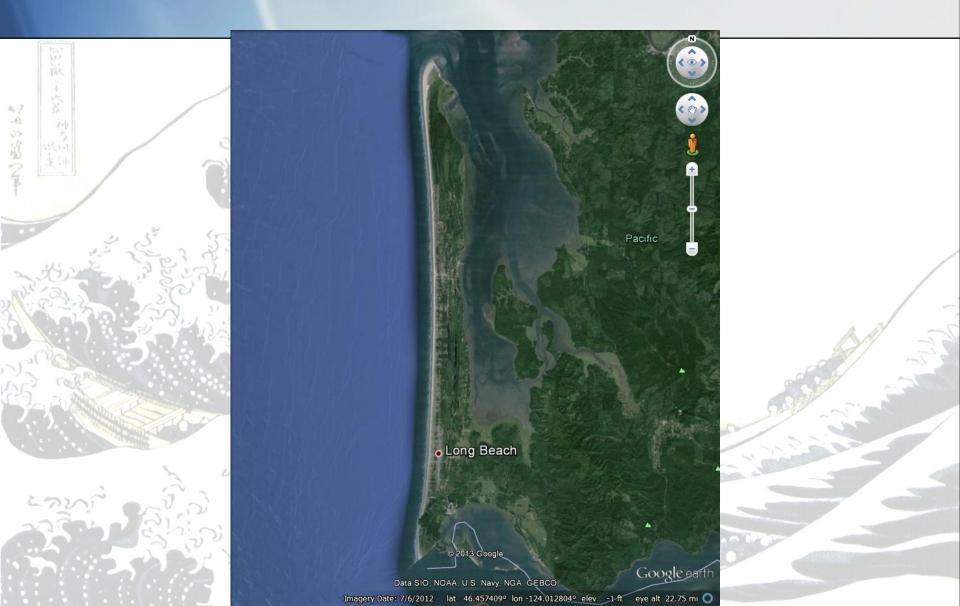
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US West Coast Population exposure to tsunami hazard

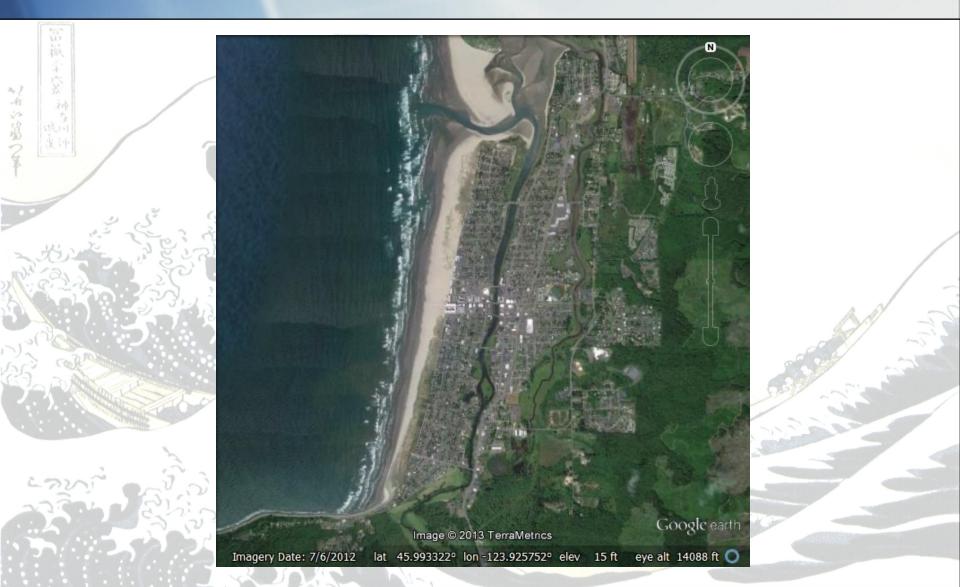
品級				
二十六景 神奈	State	Length of Coastline	Population at Risk (in evacuation zone)	
· · · · ·	California	840 miles	275,000 residents 400,000 to 2,000,000 tourist	
- 3324	Oregon	300 miles	25,000 residents 55,000 tourists	
P. 23.2	Washington	160 miles	45,000 residents 20,000 tourists	,
	Alaska	6,600 miles	105,000 residents Highly seasonal tourist count	Ι,
	Hawaii	750 miles	200,000 residents 175,000 tourists	,

Data assembled by Gary Chock, Martin & Chock, Inc.

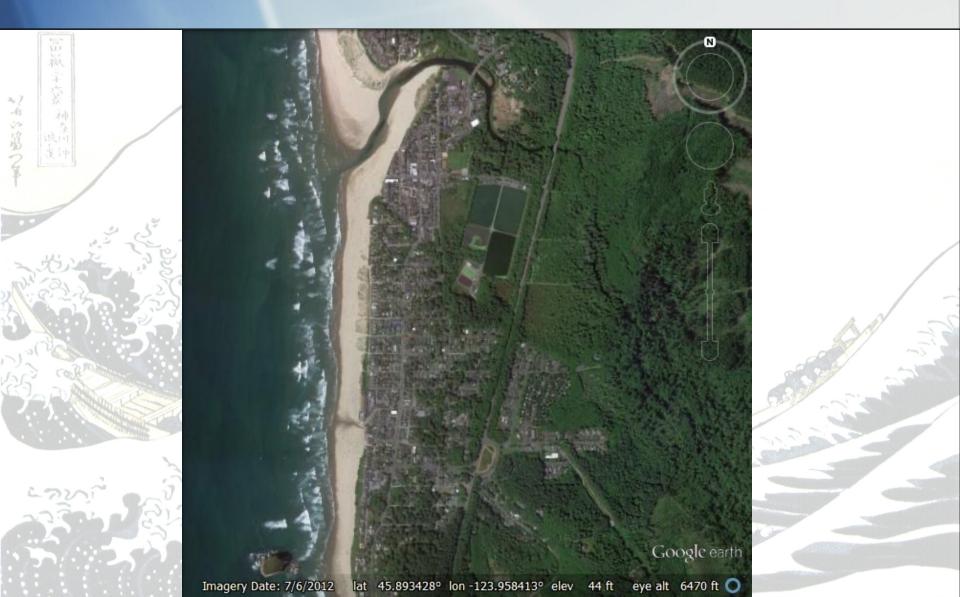
Long Beach, Washington







Cannon Beach, Oregon



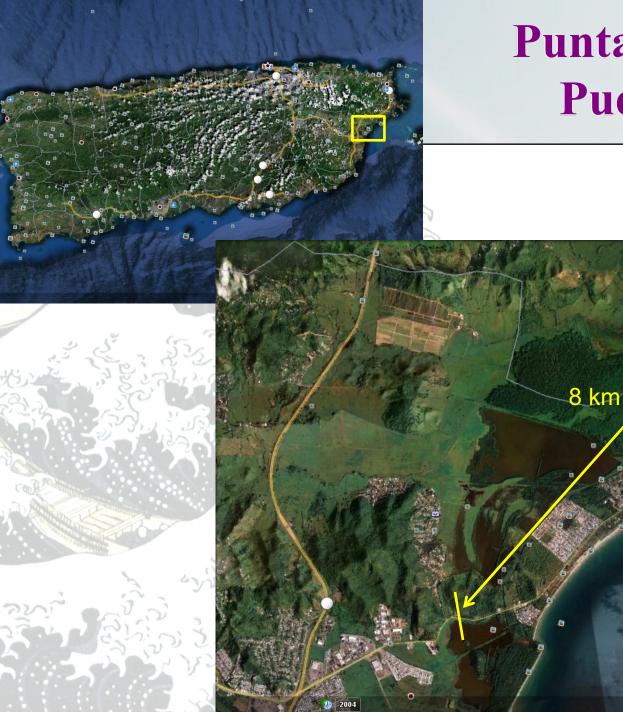
Waikiki, Hawaii





Current Evacuation Guidance

"Structural steel or reinforced concrete buildings of ten or more stories provide increased protection on or above the fourth floor"



Punta Santiago, Puerto Rico

Google earth

Punta Santiago, Puerto Rico



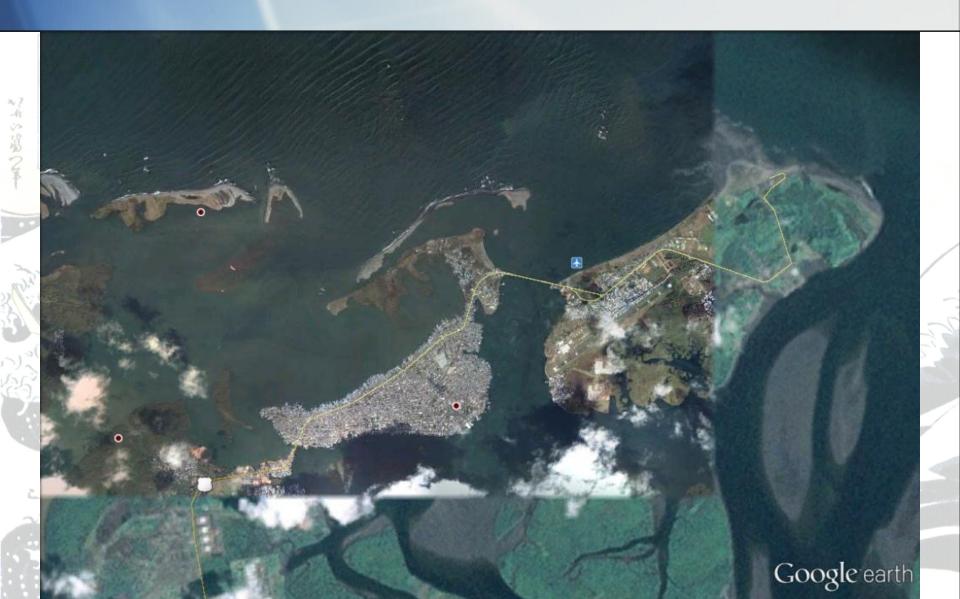
 Other structures for other neighborhoods



Tsunami Hazard in Colombia

- 2:59 AM on Dec. 12, 1979, Tumaco Earthquake
- 8.2 M_w, 33km deep
- Subduction zone between
 Nazca and South American
 Plates
- Triggered major tsunami
- First wave reached Tumaco in 3 minutes
- Estimated 600 deaths and 4000 injuries along affected coastline
- Population around 70,000

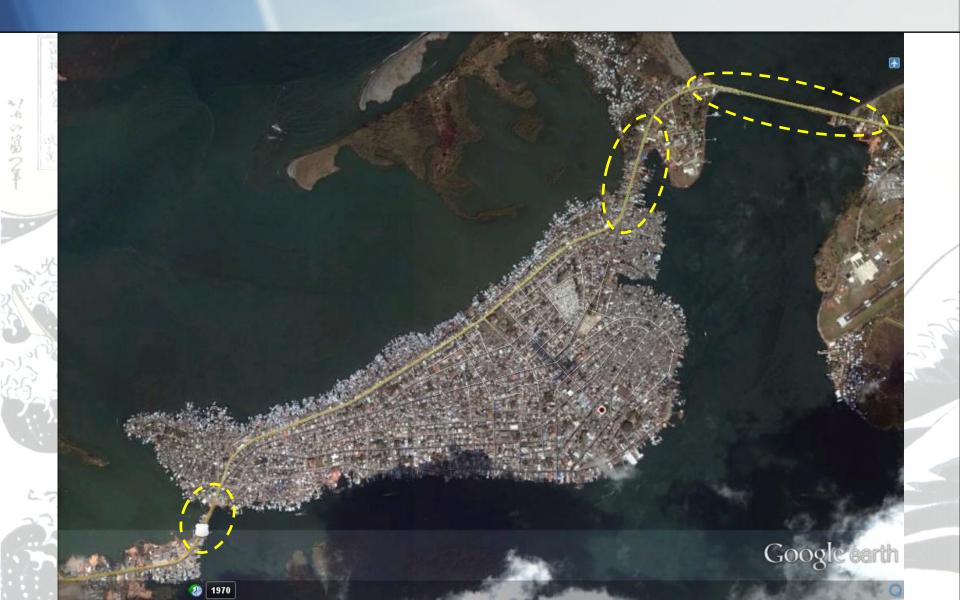
Tumaco – population 205,000



Tumaco – population 160,000



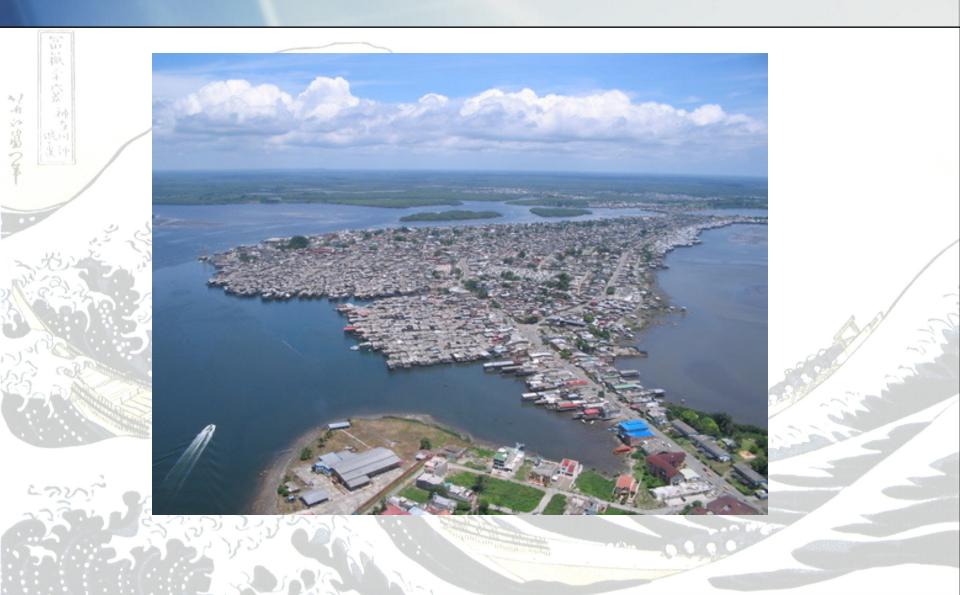
Tumaco Evacuation



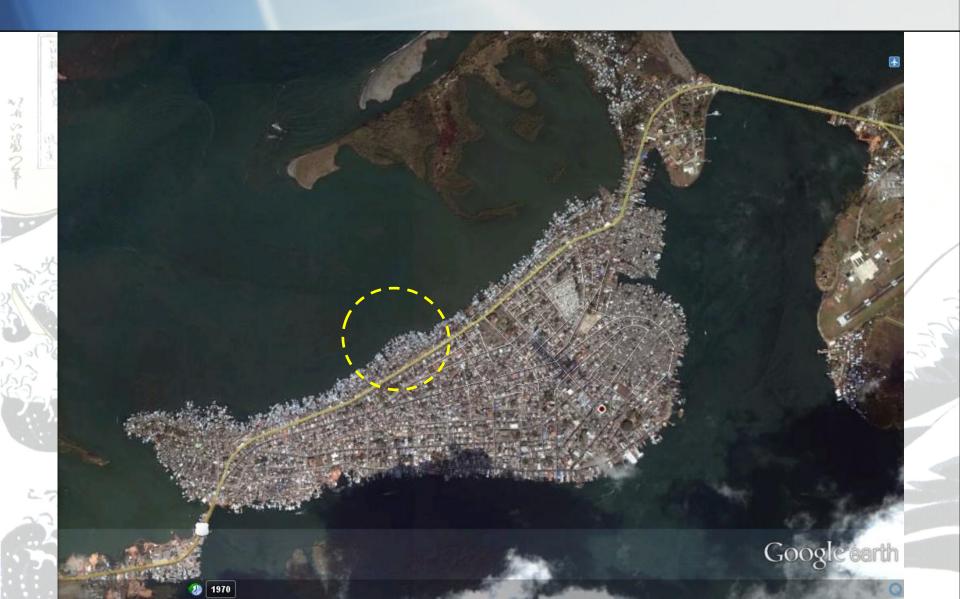
Bridge to and from Airport



Causeway to and from Airport



Tumaco Evacuation



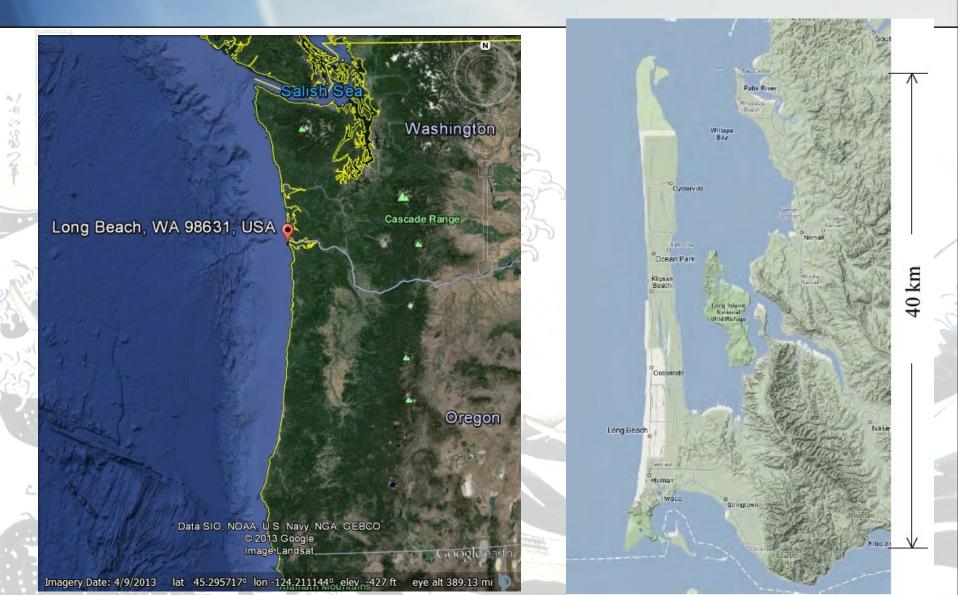
Tumaco – Typical Structures



Tumaco – Potential Vertical Evacuation Refuges from Tsunamis



Long Beach Peninsula Simulation Harry Yeh, OSU, Tim Fiez and Jonathan Karon, Gartrell Group



Long Beach Peninsula Simulation Harry Yeh, OSU, Tim Fiez and Jonathan Karon, Gartrell Group



Present Condition High Ground Only



Scenario 1 One Refuge

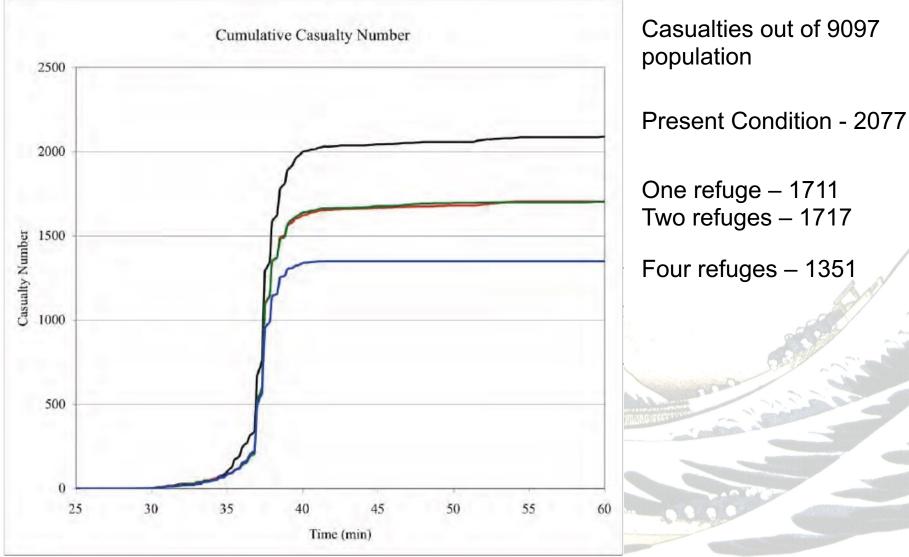


Scenario 2 Two Refuges



Scenario 3 Four Refuges

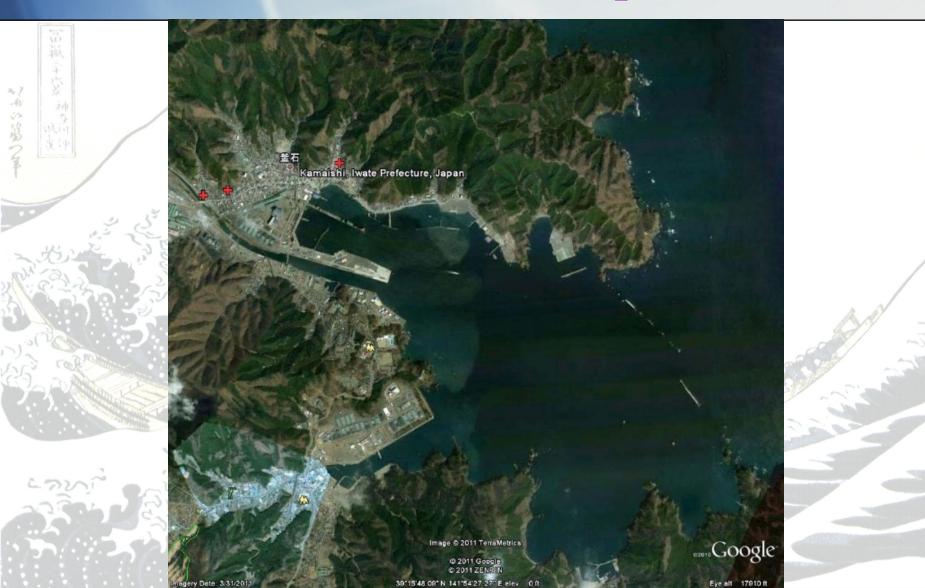
Long Beach Peninsula Simulation Harry Yeh, OSU, Tim Fiez and Jonathan Karon, Gartrell Group



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Evacuation to high ground Kamaishi Example



Evacuation to high ground Kamaishi Example

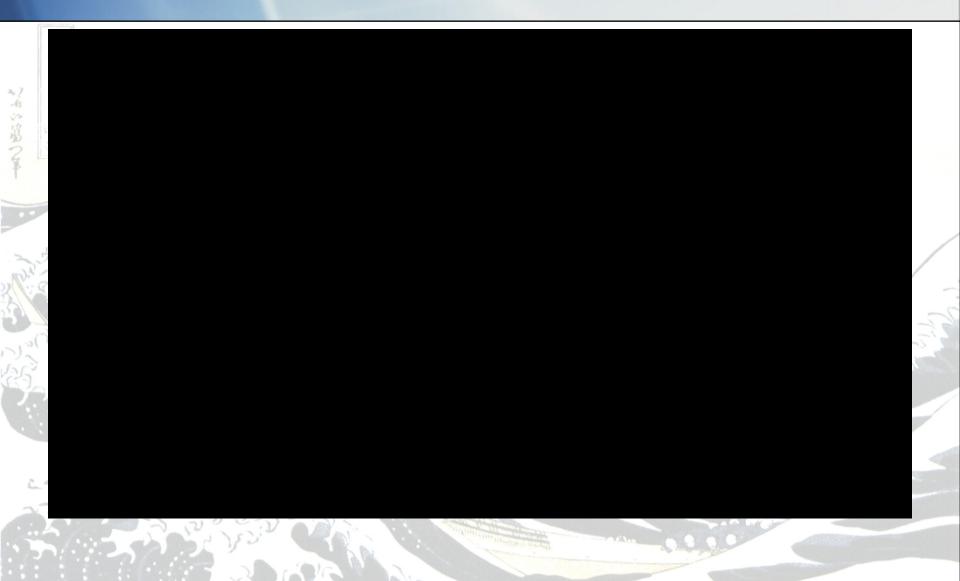


Use of Designated Tsunami Evacuation Buildings



Designated evacuation

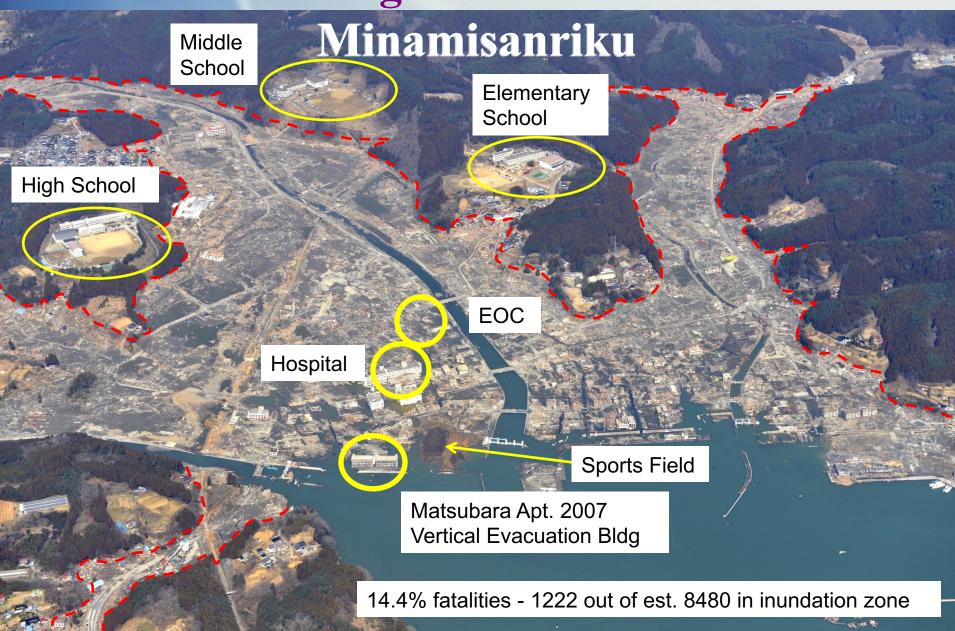
Kamaishi Survivor Video



Kamaishi Evacuation Building



Warning and Evacuation



Effective Vertical Evacuation Matsubara Community Apt. Bldg. - 2007

High-rise tsunami evacuation buildings can be effective refuges, but must be high enough!

New 4-story reinforced concrete coastal residential structure with public access roof for tsunami evacuation

Concrete building survived tsunami, but roof evacuation area inundated by 0.7m water

44 refugees, including several children, survived on roof evacuation area



Effective Vertical Evacuation Matsubara Community Apt. Bldg. - 2007

- External stair and elevator to roof refuge area
- Large refuge surrounded by secure 6ft fence





Effective Vertical Evacuation Matsubara Community Apt. Bldg. - 2007

- Significant scour around corners of building
- Collapse prevented by deep foundations

Varied Performance of Reinforced Concrete Buildings

 Varied performance of neighboring concrete buildings in Minamisanriku

Essential and Emergency Response Facilities in Harm's Way (over 300 disaster responders killed)

Minamisanriku Emergency Operations Center

Mayor Jin Sato, and 29 workers remained at center to provide live warnings during inundation





24 made it to the roof



EOC and Hospital in Background at Minamisanriku

But only Mayor Sato and 8 others survived by climbing the communication antenna and clinging to the stair guard rail. 21 emergency responders died because their vertical evacuation structure was not high enough.

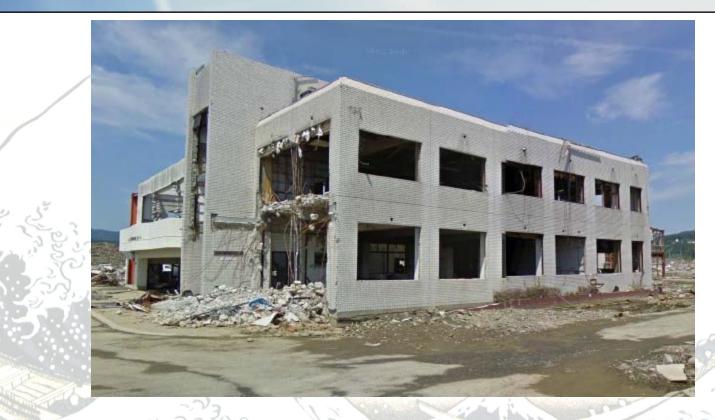
The EOC structure has been saved as a memorial to the emergency personnel who perished during the tsunami

Minamisanriku Hospital RC building with seismic retrofit

Hospital was occupied during the tsunami (320 survived) Some patients were moved to evacuation zone on roof Three stories of patient drowning fatalities (71 dead)



Minamisanriku Fisheries Cooperative



- Designated evacuation site, though only 2 floors
- Overtopped by tsunami
- Unknown number of lives lost

Arahama Elementary School, Sendai



Reinforced Concrete

Rikuzentakata

Primary School

Modern mid-rise reinforced concrete buildings with deep pile foundations generally withstood wave loads, even when nearly overtopped

Primary School – designated evacuation center. Abandoned just in time because notified by disaster officials that seawalls had been overtopped. No fatalities.

Reinforced Concrete

Rikuzentakata Primary School



Primary School – designated evacuation center. Abandoned just in time because notified by disaster officials that seawalls had been overtopped. No fatalities. Modern mid-rise reinforced concrete buildings with deep pile foundations generally withstood wave loads, even when nearly overtopped

Many Evacuation Sites Inundated



 Rikuzentakata City Hall Community Center and Gym that served as an official tsunami evacuation center was completely inundated leading to loss of life of almost all evacuees.



Cross-walks Sendai and Rikuzentakata



Sendai Crosswalk Used as unofficial refuge by 50+

Cross-walks Sendai and Rikuzentakata





Sendai Crosswalk Used as unofficial refuge by 50+

Rikuzentakata Crosswalk Almost completely destroyed – unknown casualties

Report on Performance of Evacuation Structures in Japan

- By Fraser, Leonard, Matsuo and Murakami
- GNS Science Report 2012/17
- April 2012

Tsunami evacuation: Lessons from the Great East Japan earthquake and tsunami of March 11th 2011

S. Fraser I. Matsuo G.S. Leonard H. Murakami





GNS Science Report 2012/17 April 2012

Tohoku Tsunami **ASCE/SEI Tsunami Survey Final Report**

Civil Engineering Structural Engineering





Sponsored by the Structural Engineering Institute of ASCE

On March 11, 2011, at 2:46 p.m. local time, the Great East Japan Earthquake with moment magnitude 9.0 generated a tsunami of unprecedented height and spatial extent along the northeast coast of the main island of Honshu. The Japanese government estimated that more than 250,000 buildings either collapsed or partially collapsed predominantly from the tsunami. The tsunami spread destruction inland for several kilometers, inundating an area of 525 square kilometers, or 207 square miles.

About a month after the tsunami, ASCE's Structural Engineering Institute sent a Tsunami Reconnaissance Team to Tohoku, Japan, to investigate and document the performance of buildings and other structures affected by the tsunami. For more than two weeks, the team examined nearly every town and city that suffered significant tsunami damage, focusing on buildings, bridges, and coastal protective structures within the inundation zone along the northeast coast region of Honshu.

This report presents the sequence of tsunami warning and evacuation, tsunami flow velocities, and debris loading. The authors describe the performance, types of failure, and scour effects for a variety of structures:

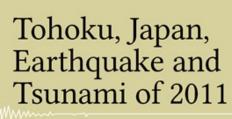
- · buildings, including low-rise and residential structures;
- · railway and roadway bridges;
- seawalls and tsunami barriers:
- breakwaters;
- · piers, quays, and wharves;
- · storage tanks, towers, and cranes.

Additional chapters analyze failure modes utilizing detailed field data collection and describe economic impacts and initial recovery efforts. Each chapter is plentifully illustrated with photographs and contains a summary of findings.

For structural engineers, the observations and analysis in this report provide critical information for designing buildings, bridges, and other structures that can withstand the effects of tsunami inundation.







東北地方日本 地震·津波 2011

Performance of Structures under Tsunami Loads













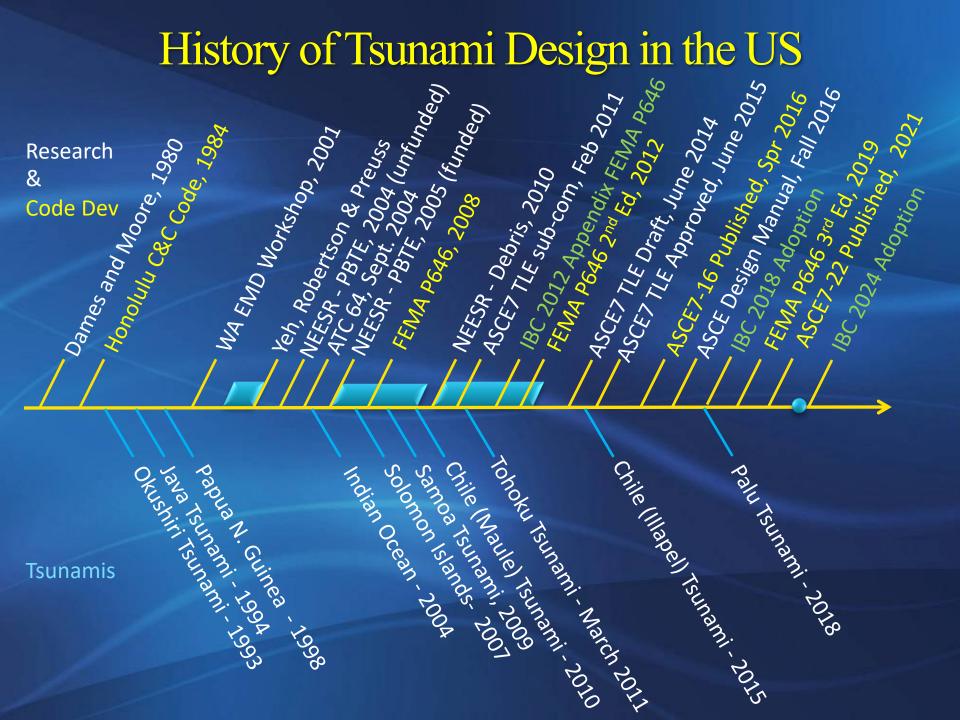
Gary Chock, S.E., Ian Robertson, S.E., David Kriebel, P.E., Mathew Francis, P.E., and Ioan Nistor, P.E.





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Guidelines for Design of Structures for Vertical Evacuation from Tsunamis (FEMA P646)

- Developed by Applied Technology Council as ATC-64 FEMA Funding First published 2008
- FEMA

- Michael Mahoney
- Robert Hanson
- ATC Management
 - Christopher Rojahn
 - Jon Heinz
 - William Holmes



Guidelines for Design of Structures for Vertical Evacuation from Tsunamis

FEMA P646 / June 2008





Guidelines for Design of Structures for Vertical Evacuation from Tsunamis (FEMA P646)

Project Team

- Steven Baldridge
- Frank Gonzalez
- Timothy Walsh
- Harry Yeh
- John Hooper
- Ian Robertson

Specifically developed for vertical evacuation buildings, not general building stock Non-mandatory language Guidelines



Guidelines for Design of Structures for Vertical Evacuation from Tsunamis

FEMA P646 / June 2008





Guidelines for Design of Structures for Vertical Evacuation from Tsunamis (FEMA P646)

Modified as ATC-79 Project Team

- Ian Robertson
- Timothy Walsh
- Harry Yeh
- John Hooper
- Gary Chock
- Revised 2012 Second Edition

Guidelines for Design of Structures for Vertical Evacuation from Tsunamis

FEMA P646 / April 2012







Vertical Evacuation Options

いかい日のころ

- Preference given to high ground
- Manmade high ground in form of mound
 - Building or other structure designed for tsunami loads

Manmade high ground Sendai Port, Japan



- Earth mounds can act as effective evacuation sites
- Must be high and large enough



Vertical Evacuation Building Designated Refuge

Port Authority Bldg.
Kesennuma, Japan
Designated as tsunami refuge
Flooded to third level
Numerous survivors sought refuge on

roof



Adjacent Building used as refuge of opportunity



Kesennuma Refuge of Opportunity

Adjacent Building used as refuge of opportunity



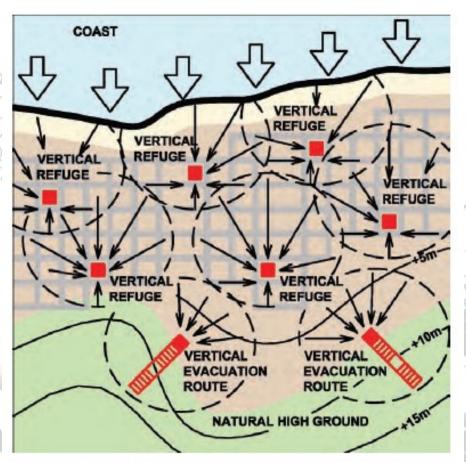
Vertical Evacuation Building Parking Garage

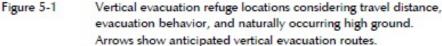
Multi-level Parking structure
Biloxi, Mississippi
Hurricane Katrina
Open to pedestrians 24 hours a day
Ramps for easy access to roof



Siting and Spacing

Provide access to high ground **Guidance on number** and location of vertical refuges Spacing is based on 2 mph walking speed and expected tsunami warning time





Siting and Spacing

Consideration given to proximity of large debris, hazardous or flammable materials May require additional precautions

É.

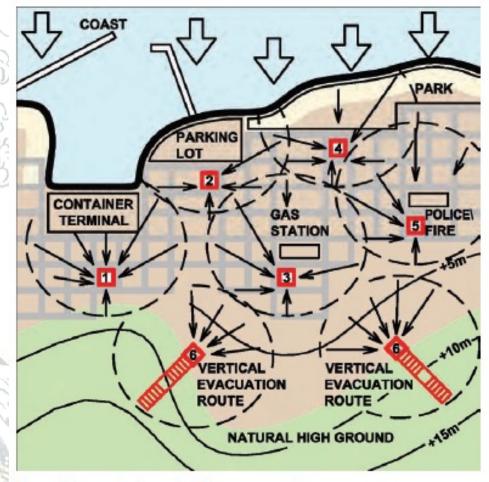


Figure 5-2 Site hazards adjacent to vertical evacuation structures (numbered locations). Arrows show anticipated vertical evacuation routes.

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- Minimum Design Loads for Buildings and Other Structures
- Referenced by the International Building Code, IBC, and therefore most US jurisdictions



- Minimum Design Loads for Buildings and Other Structures
- Chap 1 & 2 General and load combinations
- Chap 3 Dead, soil and hydrostatic loads
- Chap 4 Live loads
- Chap 5 Flood loads (riverine and storm surge)
- Chap 6 Vacant
- Chap 7 Snow loads
- Chap 8 Rain loads
- Chap 10 Ice loads
- Chap 11 23 Seismic Design
- Chap 26 31 Wind Loads

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Tsunami-Resilient Engineering Subject Matter Incorporated in ASCE 7

Scope of ASCE 7 Consensus on		
Chapter 6	Sources and Frequency	Seismic Source Assessment by USGS
 I I	Tsunami Generation Distant and Local Subduction Zones	Maps based on
I Tsunami I inundation	Open Ocean Propagation	Probabilistic Tsunami Hazard
Modeling to	Offshore Tsunami Amplitude	Analysis (PTHA)
I Define I Tsunami	Coastal Inundation and Flow Velocities	I
Design Zones	Fluid-Structure Interaction	
Loads and Effects	Structural Loading	
incorporating	Structural Response	
Coastal, Hydraulic,	Scour and Erosion	
Structural, and Geotechnical Engineering	Performance by Risk Category	Structural Reliability Validated
	Consequences (Life and economic losses)	Societal Impact Assessment for
	Warning and Evacuation Capability	the Five Western States by USGS

ASCE 7 Chapter 6- Tsunami Loads and Effects

- 6.1 General Requirements
- 6.2-6.3 Definitions, Symbols and Notation
- 6.4 Tsunami Risk Categories
- 6.5 Analysis of Design Inundation Depth and Velocity
- 6.6 Inundation Depth and Flow Velocity Based on Runup
- 6.7 Inundation Depth and Flow Velocity Based on Site-Specific Probabilistic Tsunami Hazard Analysis
- 6.8 Structural Design Procedures for Tsunami Effects
- 6.9 Hydrostatic Loads
- 6.10 Hydrodynamic Loads
- 6.11 Debris Impact Loads
- 6.12 Foundation Design
- 6.13 Structural Countermeasures for Tsunami Loading
- 6.14 Tsunami Vertical Evacuation Refuge Structures
- 6.15 Designated Nonstructural Systems
- 6.16 Non-Building Structures

Consequence Guidance on Risk Categories of Buildings Per ASCE 7

Risk Category I	Up to 2 persons affected	
	(e.g., agricultural and minor storage facilities, etc.)	
Risk Category II	Approximately 3 to 300 persons affected	
(Tsunami Design	(e.g., Office buildings, condominiums, hotels, etc.)	
Optional)		
Risk Category III	Approximately 300 to 5,000+ affected	
(Tsunami Design		
Required)	(e.g., Public assembly halls, arenas, high occupancy educational facilities, public utility facilities, etc.)	
Risk Category IV	Over 5,000 persons affected	
(Tsunami Design		
Required)	(e.g., hospitals and emergency shelters, emergency operations centers, first responder facilities, air traffic control, toxic material storage, etc.)	

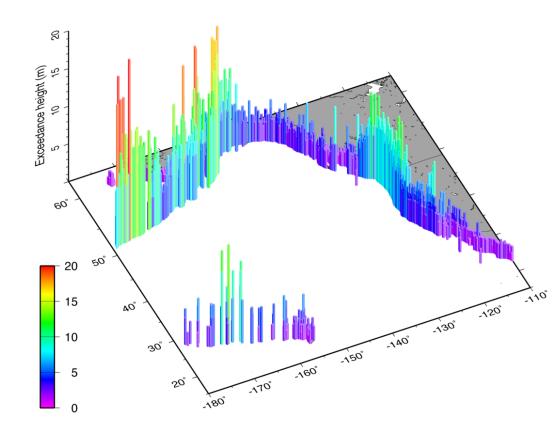
Risk Category II Buildings – Determined by Local Code Adoption

- The state or local government has the option to determine a threshold height for where tsunamiresilient design requirements for Risk Category II buildings.
- The threshold height would depend on the community's tsunami hazard, tsunami response procedures, and whole community disaster resilience goals.
- When evacuation travel times exceed the available time to tsunami arrival, there is a greater need for vertical evacuation into an ample number of sufficiently tall Category II buildings.

Tsunami Design Zone: Lessons from the Tohoku, Chile, and Sumatra Tsunamis

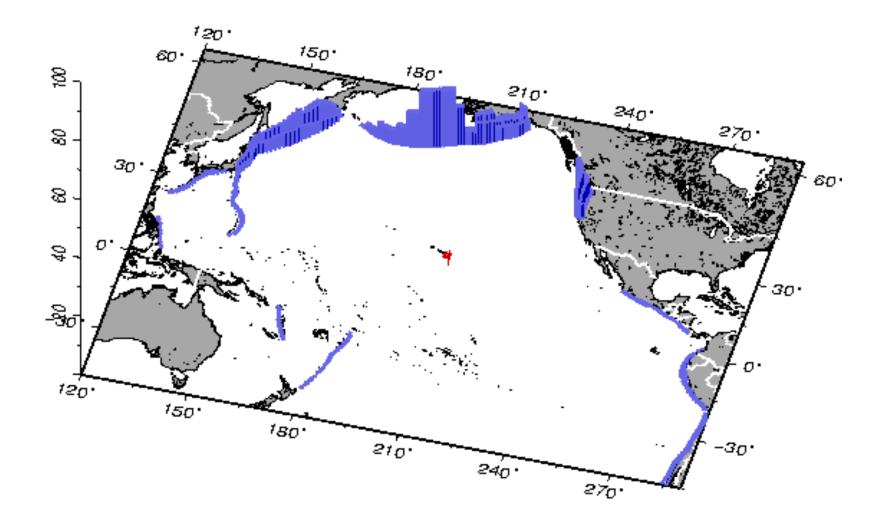
- <u>Recorded history may not</u> provide a sufficient measure of the potential heights of great tsunamis.
- Design must consider the occurrence of events greater than in the historical record
- Therefore, probabilistic physics-based Tsunami Hazard Analysis should be performed in addition to historical event scenarios
- This is consistent with the probabilistic seismic hazard analysis

Exceedance waveheights: 2500 yr

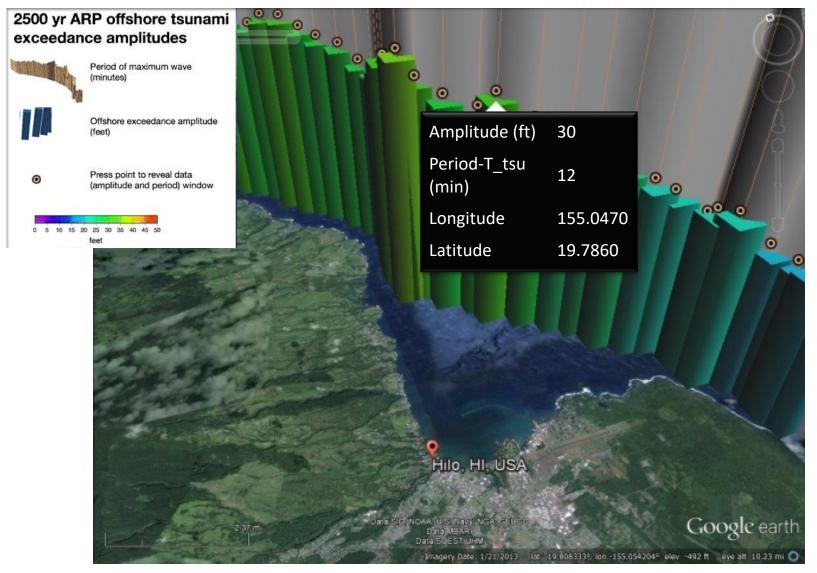


Disaggregated Hazard for Hilo, HI

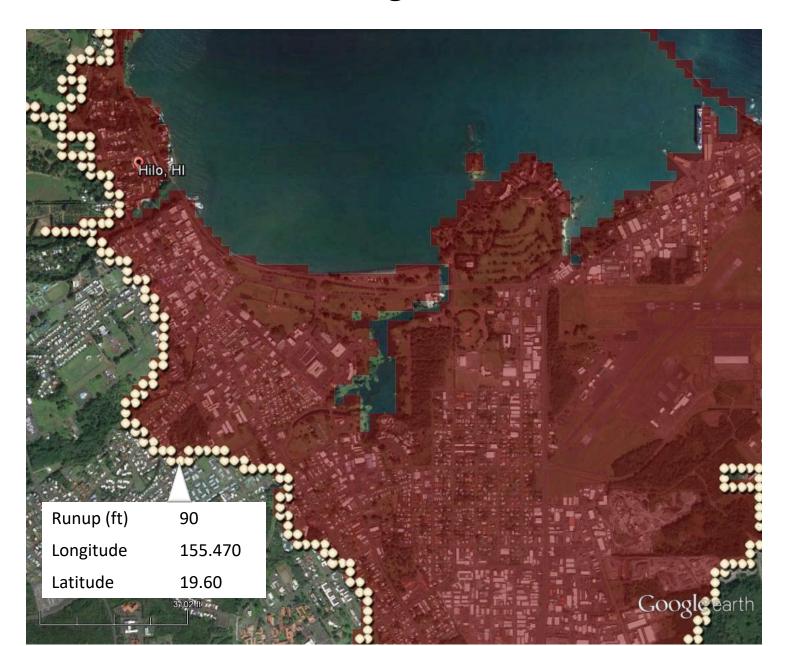
• Sources: Aleutian, Alaska, and Kamchatka-Kurile



Offshore Tsunami Amplitude and Period for the Maximum Considered Tsunami at Hilo Harbor, HI



Tsunami Design Zone - Hilo



ASCE 7 Chapter 6- Tsunami Loads and Effects

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- 6.14 Tsunami Vertical Evacuation Refuge Structures
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Structural Loads



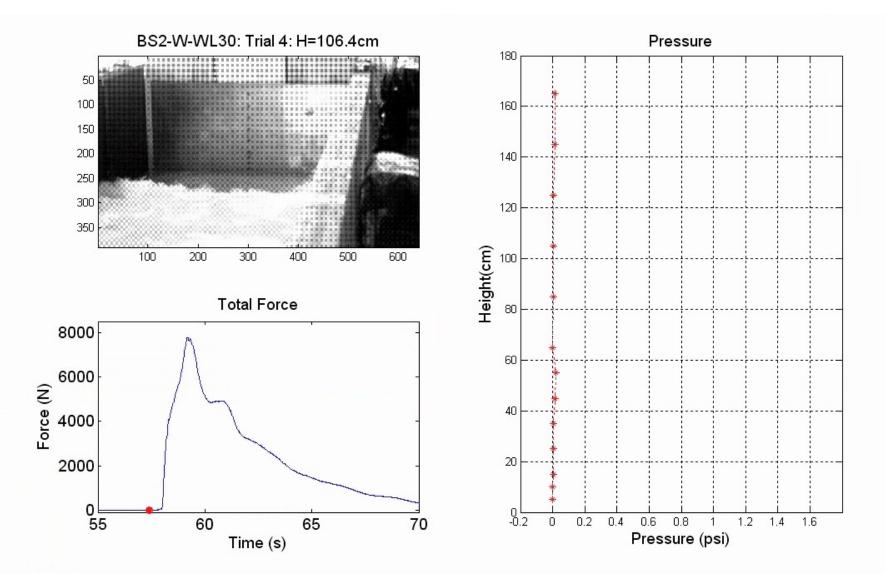
Tsunami Loads and Effects

- Hydrostatic Forces (equations of the form $k_s \rho_{sw} gh$)
 - Unbalanced Lateral Forces at initial flooding
 - Buoyant Uplift based on displaced volume
 - Residual Water Surcharge Loads on Elevated Floors
- Hydrodynamic Forces (equations of the form $\frac{1}{2} k_s \rho_{sw}(hu^2)$
 - Drag Forces per drag coefficient C_d based on size and element
 - Lateral Impulsive Forces of Tsunami Bores on Broad Walls: Factor of 1.5
 - Hydrodynamic Pressurization by Stagnated Flow per Benoulli
 - Shock pressure effect of entrapped bore
- Waterborne Debris Impact Forces (flow speed and \sqrt{k} m)
 - Poles, passenger vehicles, medium boulders always applied
 - Shipping containers, boats if structure is in proximity to hazard zone
 - Extraordinary impacts of ships only where in proximity to Risk Category III & IV structures
- Scour Effects (mostly prescriptive based on flow depth)

NEESR – Development of Performance Based Tsunami Engineering, PBTE



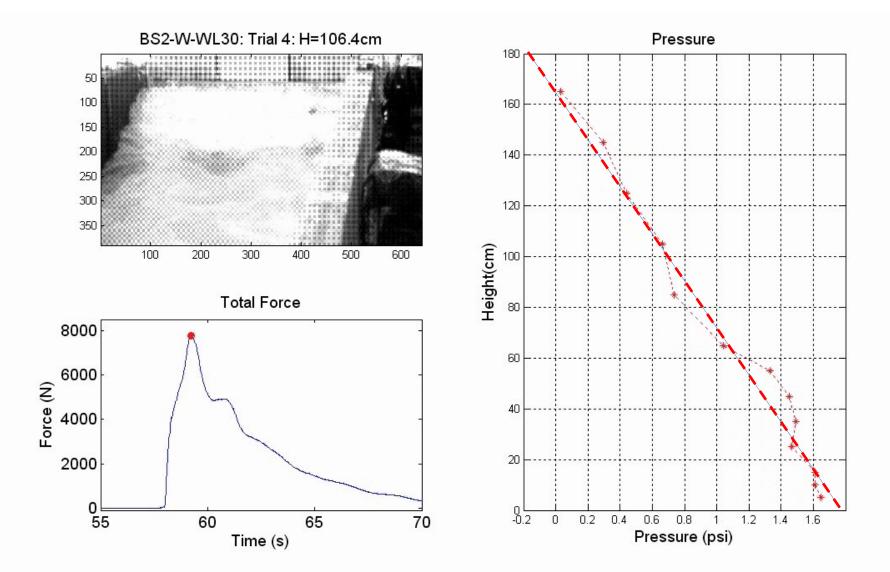
NEESR – Development of Performance Based Tsunami Engineering, PBTE



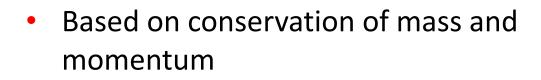
NEESR – Development of Performance Based Tsunami Engineering, PBTE



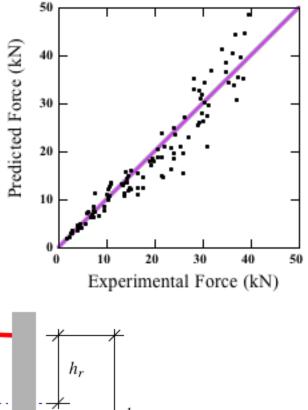
NEESR - Structural Loading Direct Bore Impact on Solid Wall

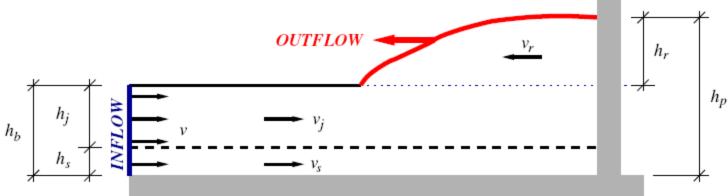


Hydrodynamic Force on Wall due to Bore Impact



$$F_{w} = \rho_{sw} \left(\frac{1}{2} g h_{b}^{2} + h_{j} v_{j}^{2} + g^{\frac{1}{3}} (h_{j} v_{j})^{\frac{4}{3}} \right)$$





Sendai Bore Strike on R/C Structure



Sendai Bore Strike on R/C Structure



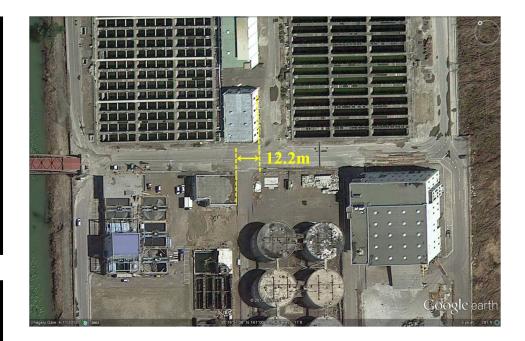
Velocity Analysis



Frame 260 – First Building Impact



Frame 316 – Second Building Impact



Video rate of 30 fps Time from Frame 260 to 316 = 1.87 sec. Distance between buildings = 12.2 m Bore velocity = 12.2/1.87 = 6.5 m/s Jump height approx. 5.5m over approx. 0.5m standing water

Bore Strike on R/C Structure

Minami Gamou Wastewater Treatment Plant - subjected to direct bore impact



Structural drawings obtained from the Wastewater Treatment Plant

Bore Strike on R/C Structure



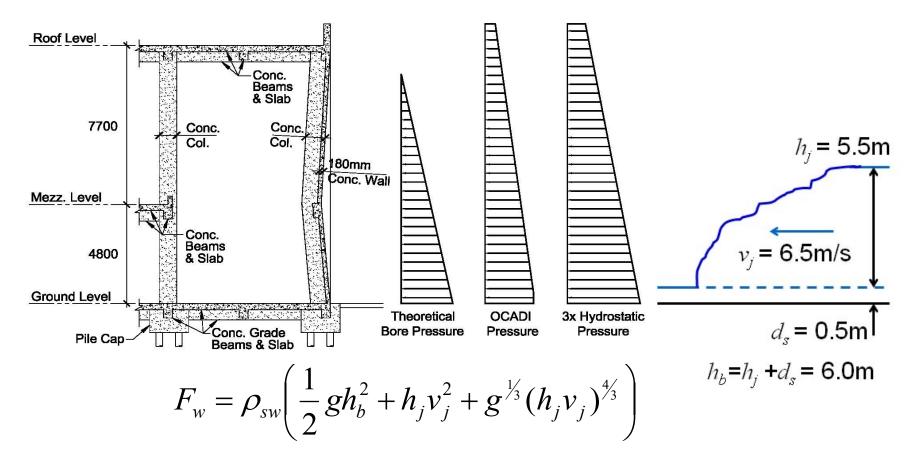
Interior view of 2-story wall

Lidar scan of 2-story wall

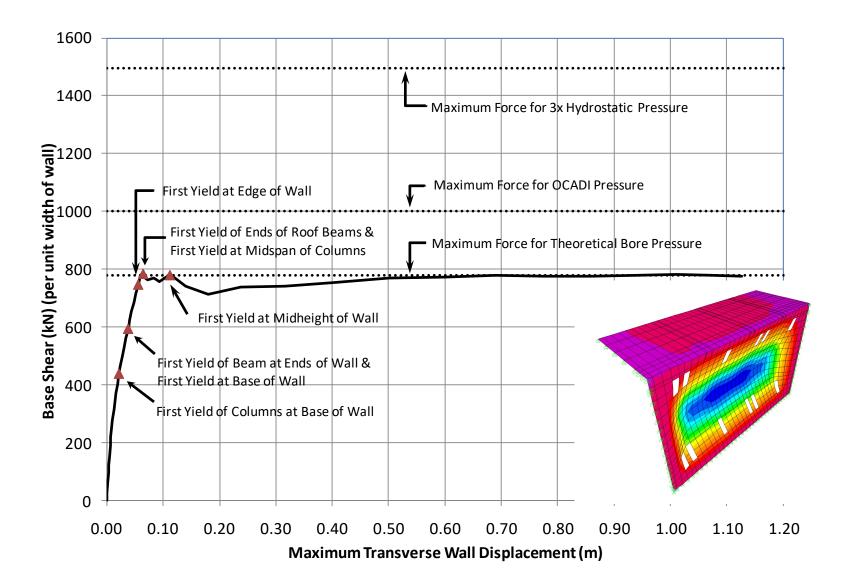
Minami Gamou Wastewater Treatment Plant

Bore Impact Forces Minami Gamou Treatment Plant

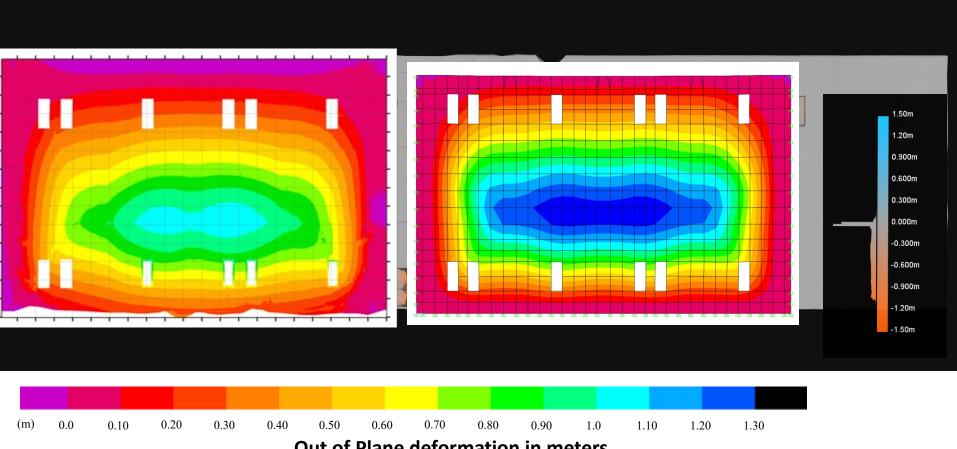
 Comparison with Different Bore Pressures used in Japan Tsunami Standards



Bore Impact Forces Non-linear Finite Element Analysis



FEA compared with Lidar scan



Out of Plane deformation in meters

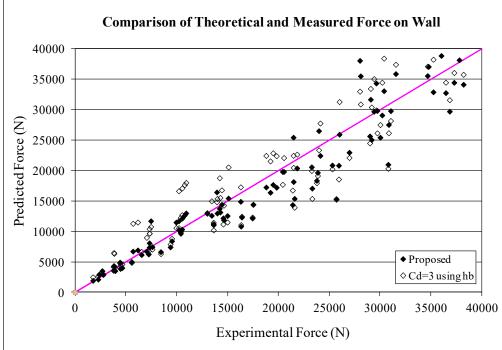
Minami Gamou Wastewater Treatment Plant - subjected to direct bore impact

Simplified Equation for Impulse Load

$$F_{w} = \rho_{sw} \left(\frac{1}{2} g h_{b}^{2} + h_{j} v_{j}^{2} + g^{\frac{1}{3}} (h_{j} v_{j})^{\frac{4}{3}} \right)$$

 Apply a factor of 1.5 to the conventional drag force, but as a uniform load rather than as a triangular load

$$F_d = 1.5(\frac{1}{2}k_s\rho_{sw}C_dbhu^2)$$



Types of Floating Debris Logs and Shipping Containers







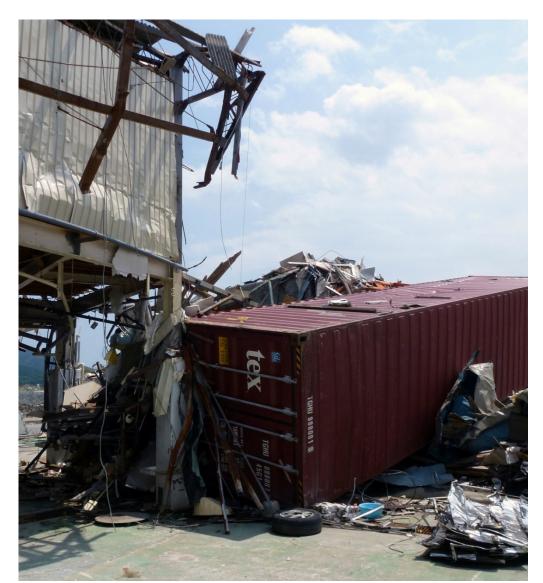


Shipping Container Debris



Talcahuano harbor area four days after the Feb 27 2010 Chile tsunami

Shipping Containers





(Samoa)

(Japan)

Types of Rolling Debris Rocks and Concrete Debris







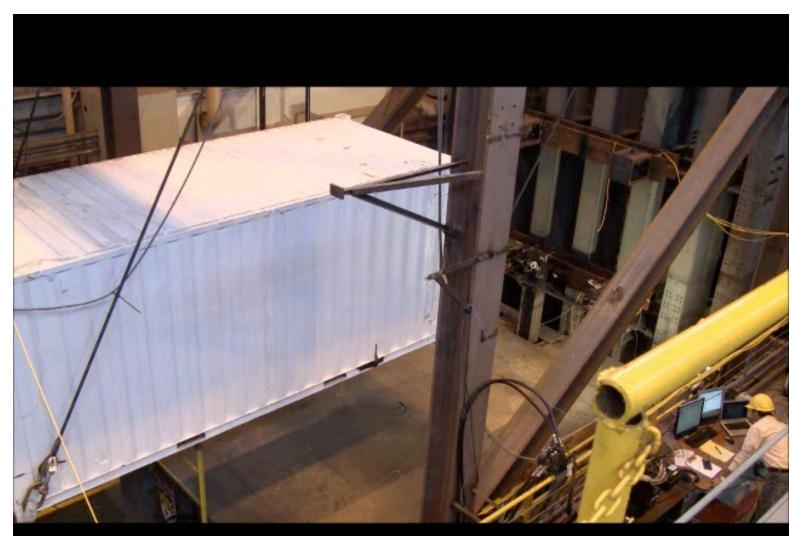
ISO 20-ft Shipping Container

- 6.1 m x 2.4 m x 2.6 m and 2300 kg empty
- Containers have 2 bottom rails and 2 top rails
- Pendulum setup; longitudinal rails strike load cell(s)

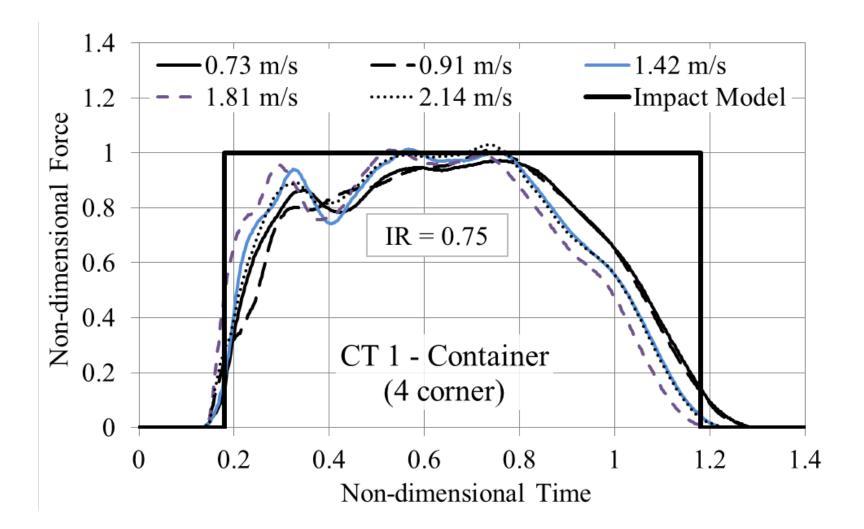


Shipping Container Impact



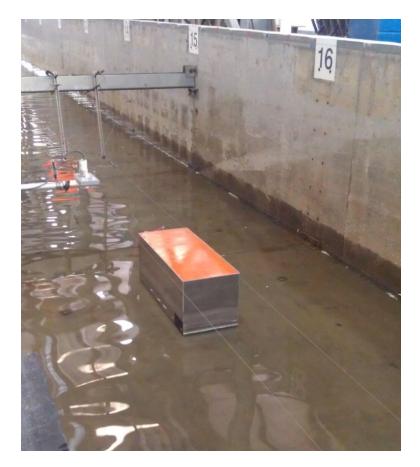


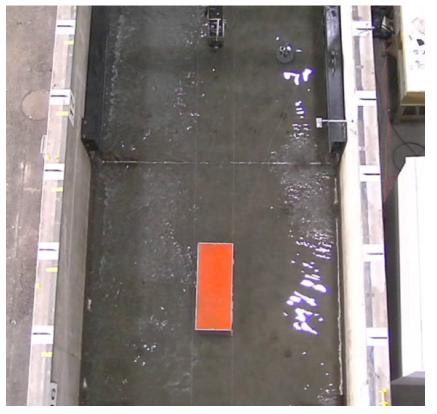
Impact Force Time History



Aluminum and Acrylic Containers

- 1/5 scale model containers of aluminum and acrylic
- Guide wires controlled the trajectory
- Container hits underwater load cell to measure the force



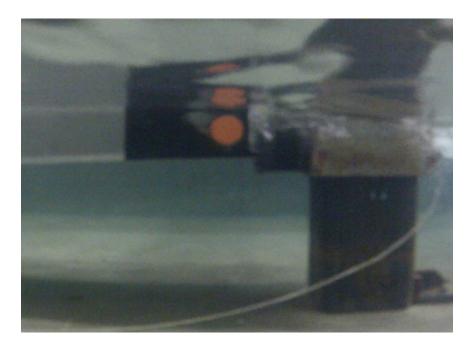


Column and load cell at top of photo

Impact with Load Cell

- In-air tests carried out with pendulum set-up for baseline
- In-water impact filmed by submersible camera
- Impact was on bottom plate to approximate longitudinal rail impact





In-air impact

In-water impact

Container Impact

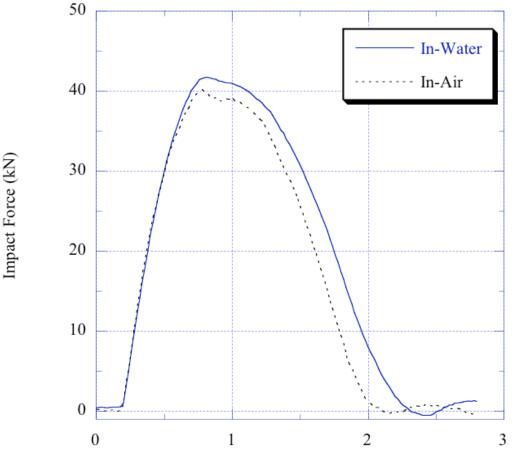


Side View



Force Time-History

- In-water impact and in-air impact very similar
 - Less difference between in-air and in-water compared to scatter between different in-water trials



Time (msec)

Debris Impact Force

• Nominal maximum impact force

$$F_{ni} = u_{max} \sqrt{km_d}$$

- Factored design force based on importance factor $F_i = I_{TSU}F_{ni}$
- Impact duration

$$t_d = \frac{2m_d u_{max}}{F_{ni}}$$

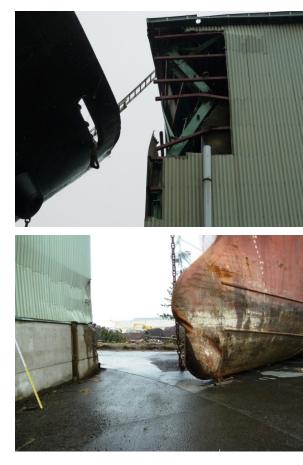
- Force capped based on strength of debris
 - Shipping Container: $F_i = 330C_o I_{TSU}$
 - Wooden Log: $F_i = 165C_o I_{TSU}$
 - Where: $C_o = 0.65$, Impact orientation factor
- Contents increase impact duration but not force

Impact induced Progressive Collapse



Ship Impact – Sendai Port





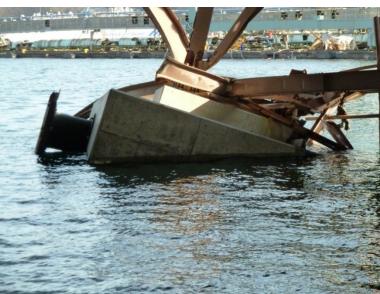
Ship Impact damage - Kamaishi



Damage to pier and warehouse due to multiple impacts from single loose ship







Kamaishi Pier



 Two survivor videos show evidence of ship impact on blue warehouse

Kamaishi Ship Impact



Ship Velocity





$$\Delta t = \frac{(1805 - 1666)}{30 \, fps} = 4.63s$$

:
$$v = \frac{33m}{4.63s} = 7.13m/s = 25.6kph$$

Ship Impact in Kamaishi Port



Ship impact damage to steel framed building on piled foundations in Kamaishi

Damming of Waterborne Debris





Three-Story Steel MRF collapsed and pushed into concrete building

Three-Story Steel MRF with 5 meters of debris load accumulation wrapping

Damming of Waterborne Debris

$$F_{dm} = \frac{1}{2} \rho_s C_d B_d (hu^2)_{\text{max}}$$

Where $B_d = 40$ feet or one structural bay



Hurricane Katrina, 2005

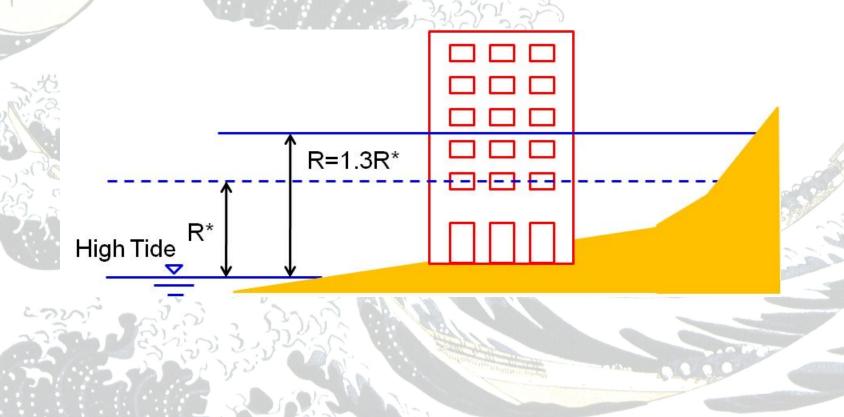
 Recommends refuge elevation be 1 story (3m, 10ft) above predicted inundation (with 1.3 uncertainty factor)

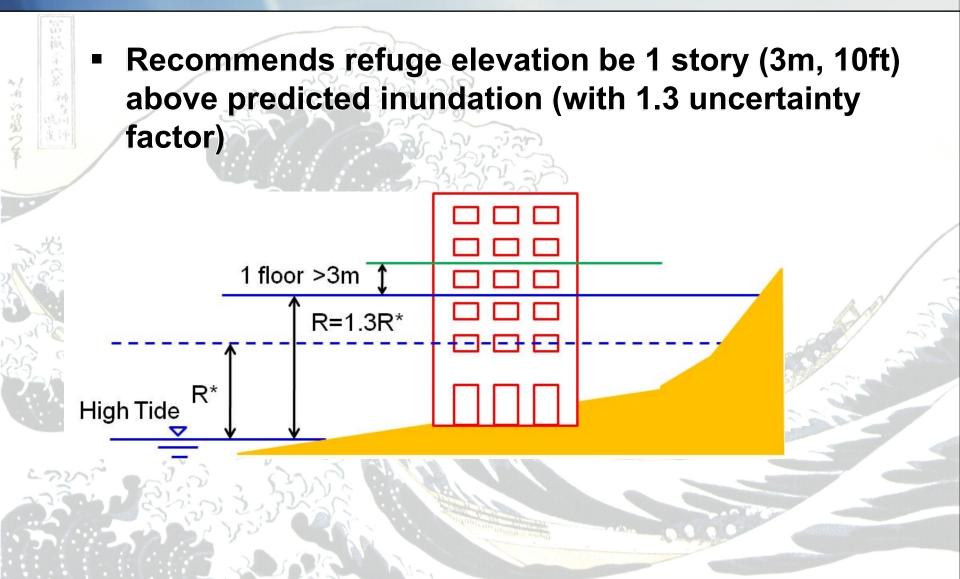
High Tide

 Recommends refuge elevation be 1 story (3m, 10ft) above predicted inundation (with 1.3 uncertainty factor)

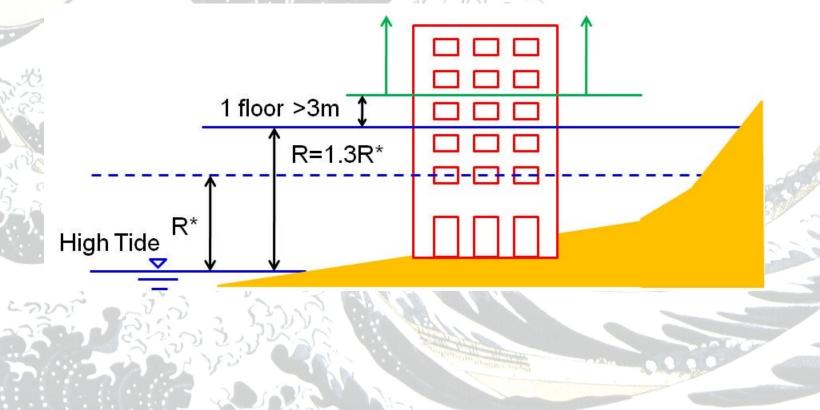


 Recommends refuge elevation be 1 story (3m, 10ft) above predicted inundation (with 1.3 uncertainty factor)





 Recommends refuge elevation be 1 story (3m, 10ft) above predicted inundation (with 1.3 uncertainty factor)



FEMA P646 Third Edition

FEMA funding to update P-646 Remove loading expressions

- Combine with P-646A, community planning guide
- Retrofit of Existing Structures
 - Quality Assurance for Vertical Evacuation Structures – Peer Review
- Planning considerations
- 24/7 Access and Entry
- Disabled access (ADA)
- Elevation of critical equipment
 - Cost considerations and financing



Guidelines for Design of Structures for Vertical Evacuation from Tsunamis Third Edition

FEMA P-646 / August 2019





ASCE Tsunami Design Guide

Tsunami design guide published by ASCE in 2020 with numerous design examples.

Tsunami Loads and Effects

Guide to the Tsunami Design Provisions of ASCE 7-16 Jan N. Robertson, Ph.D., S.E.

ASCE

Outline

- Need for Vertical Evacuation Refuges
 - Performance of Vertical Evacuation Refuges during Tohoku Tsunami
- FEMA P-646 design guidelines
 - **ASCE-7 Tsunami Loads and Effects chapter**
- Vertical Evacuation Refuge designs in US
- Conclusions

Cannon Beach Experience



Cannon Beach City Hall/TEB conceptual Design – Ecola Architects, PC (2008)

Vertical Evacuation Refuges built to ASCE 7-16



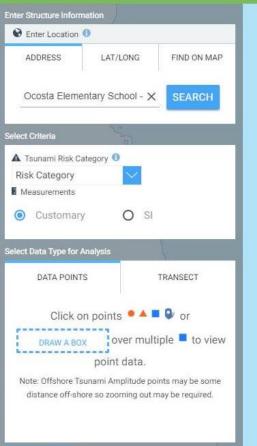


 OSU Hatfield Marine Science Building

Newport, WA

Ocosta Elementary School, Westport, Washington

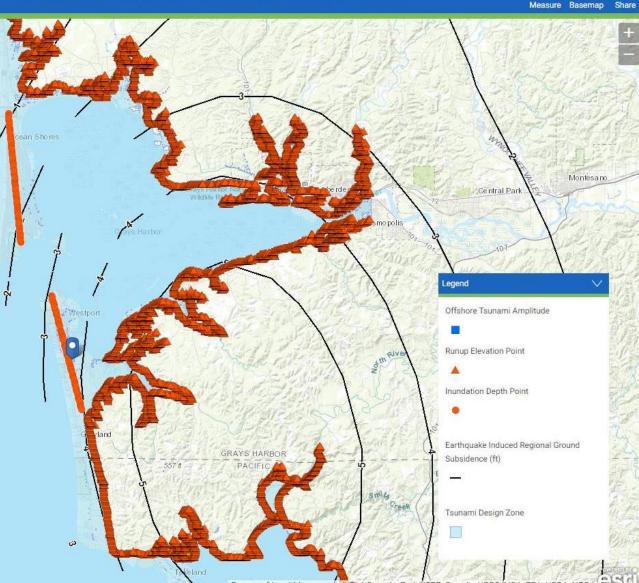
ASCE TSUNAMI Hazard Tool ASCE TSUNAMI Design Geodatabase Version 2016-1.0



CLEAR MAP

© 2017

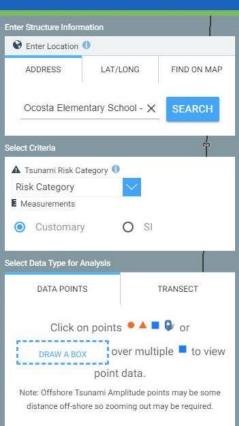
All data are per the requirements of the ASCE/SEI 7 standard; local requirements may vary.



Ocosta Elementary School, Westport, Washington

ASCE TSUNAMI Hazard Tool ASCE TSUNAMI Design Geodatabase Version 2016-1.0

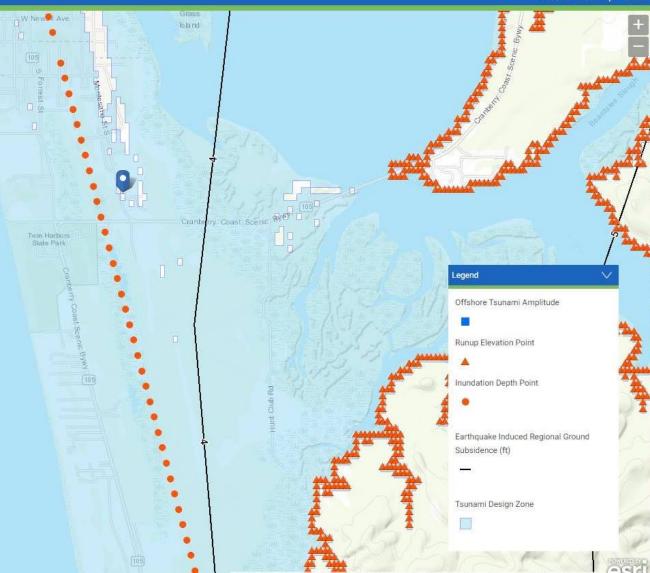
Measure Baseman



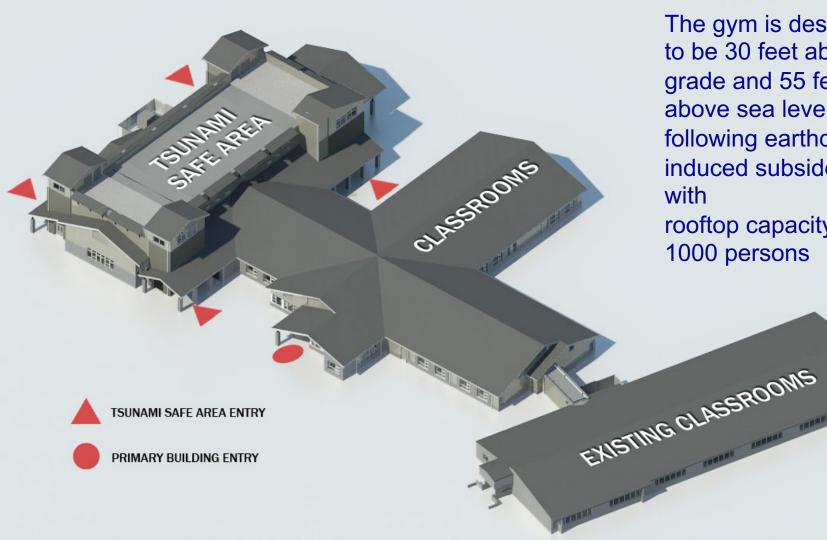
CLEAR MAP

6 2017

All data are per the requirements of the ASCE/SEI 7 standard; local requirements may vary.

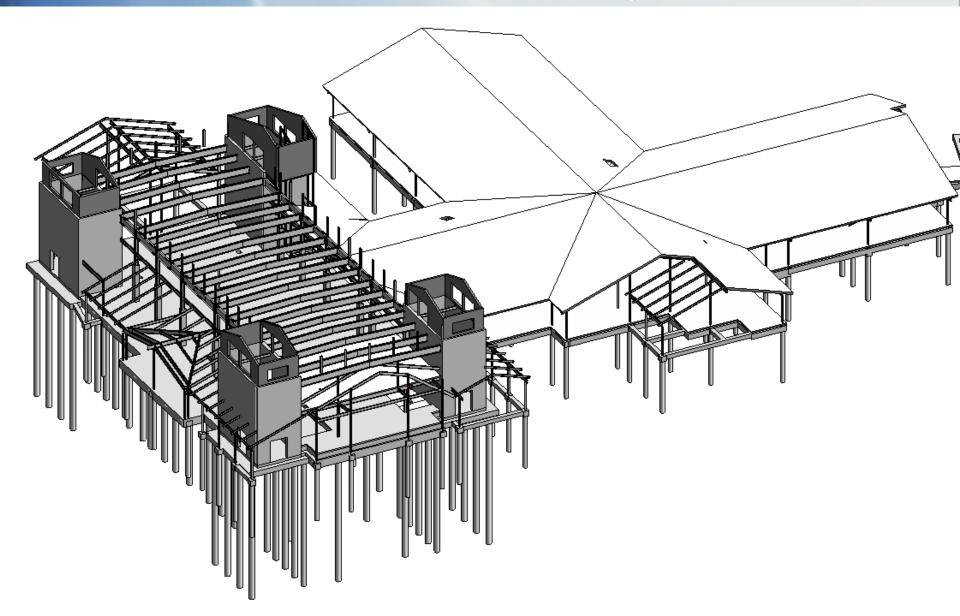


Ocosta Elementary School Westport, Washington America's first tsunami refuge

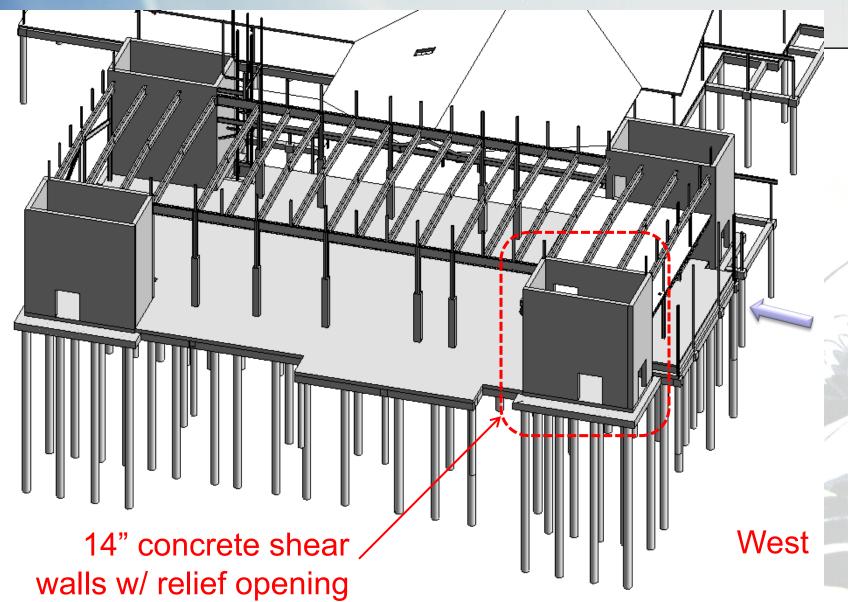


The gym is designed to be 30 feet above grade and 55 feet above sea level following earthquakeinduced subsidence, rooftop capacity for 1000 persons

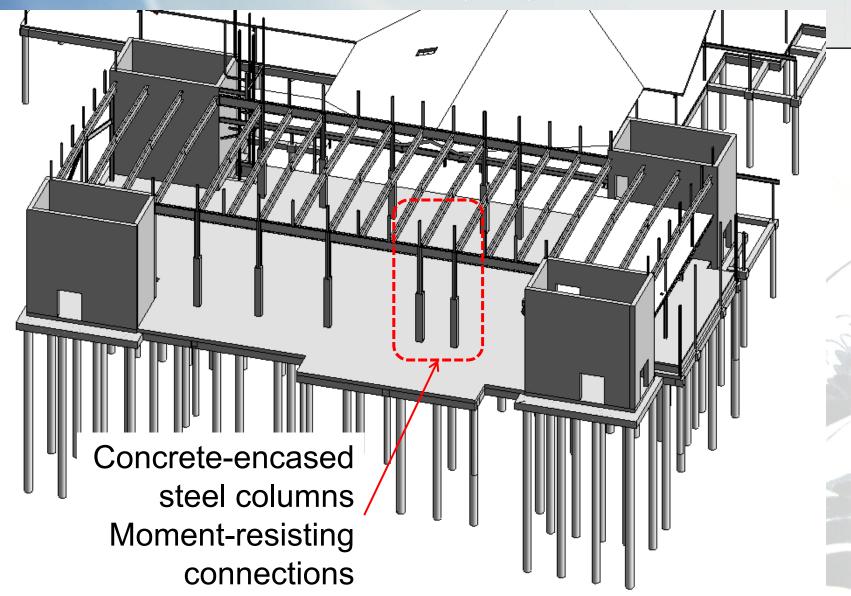
Foundation Design



Structural Lateral System



Structural Gravity System

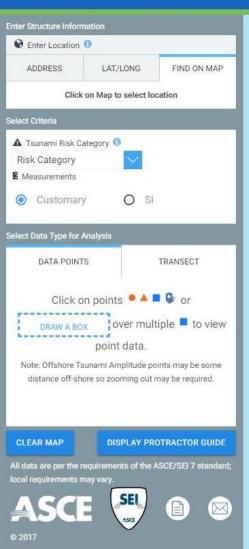


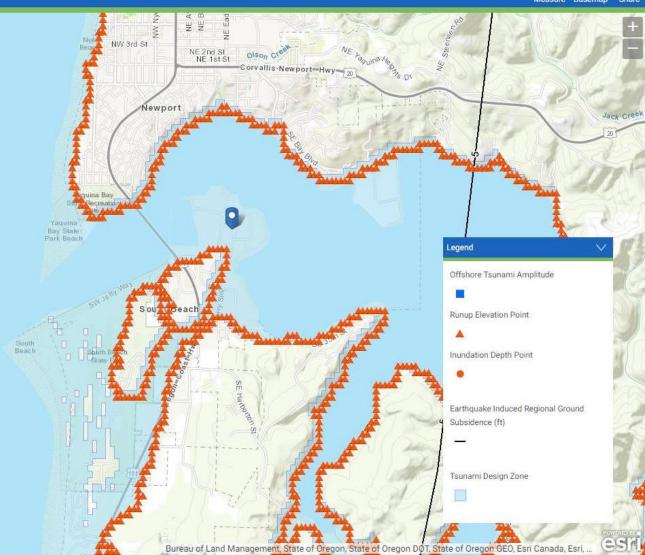
Ocosta Elementary School Westport, Washington



OSU Hatfield Marine Science Center, Newport, Oregon, USA

ASCE TSUNAMI HAZARD TOO ASCE TSUNAMI Design Geodatabase Version 2016-1.0





OSU Hatfield Marine Science Center, Newport, Oregon, USA



OSU Hatfield Marine Science Center, Newport, Oregon, USA



Conclusions

- With natural hazards, history does not repeat itself
- Probabilistic Tsunami Hazard Analysis is the basis for the development of 2500-yr Tsunami Design Zone maps.
- The ASCE 7 provisions constitute a comprehensive method for reliable tsunami structural resilience, making tsunamis a required consideration for design of structures in the five western states.
- Specified design procedures are provided for all possible loading conditions
- Coastal communities and cities are also encouraged to require tsunami design for taller Risk Category II buildings, in order to provide a greater number of taller buildings that will be life-safe and disaster-resilient.
- FEMA P-646 provides planning guidance for communities developing Vertical Evacuation Refuges for Tsunamis (VERTs)

Thank You!



Any Questions?



Thank-You

